

32 BAGDAD STREET, REGENTS PARK

PROPOSED CHILDCARE CENTRE

TRAFFIC & PARKING IMPACT ASSESSMENT

SEPTEMBER 2023

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32 BAGDAD STREET, REGENT PARK
PROPOSED CHILDCARE CENTRE
DATE: 22 SEPTEMBER 2023

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Document Management

Traffic & Parking Assessment – 32 Bagdad Street, Regents Park

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Table of Contents

1	INTF	RODUCTION	4
2	EXIS	STING SITE DESCRIPTION	5
3	EXIS	STING TRAFFIC CONDITIONS	7
	3.1	Road Network and Classification	7
	3.2	Road Description and Traffic Control	7
	3.3	Current Traffic Flows	9
	3.4	Existing Transportation Services	. 13
4	PRO	POSED DEVELOPMENT	.14
	4.1	Description of the proposal	.14
	4.2	Vehicular & Pedestrian Access	.15
	4.3	On-site Parking Provision	.16
	4.4	On-site Parking Layout and Circulation	. 17
	4.5	Waste Collection & Deliveries	. 18
5	ON-S	STREET PARKING PROVISION	. 19
	5.1	Existing Parking Controls	. 19
	5.2	Impacts of Proposed Development on Parking	.19
6	EXT	ERNAL TRAFFIC IMPACT	20
	6.1	Estimated Future Traffic Generation	20
	6.2	Projected Intersection Performance	20
7	CON	ICLUSION	28
	Append	lix A – Proposed Development Plans	29
	Append	lix B – Vehicle Swept Paths	. 33
	Append	lix C – SIDRA Intersection Analysis	48



1 INTRODUCTION

This report has been prepared by Hemanote Consultants to assess the traffic and parking implications of the proposed childcare centre to be located at **32 Bagdad Street, Regents Park**, accommodating up to 48 children places between the ages of 0 to 6 years old.

This report is to be read in conjunction with the architectural plans prepared by Boris Grgurevic and Associates (reduced copy of the plans is attached in Appendix 'A' of this report) and submitted to Canterbury-Bankstown Council as part of a Development Application.

This report is set as follows:

- Section 2: Description of the existing site location and its use;
- Section 3: Description of existing traffic conditions near the subject site;
- Section 4: Description of the proposal, vehicular access, on-site parking provision, layout and circulation;
- Section 5: Assessment of impacts on parking;
- Section 6: Assessment of impacts on traffic in the vicinity of the subject site; and
- Section 7: Outlines conclusions.



2 EXISTING SITE DESCRIPTION

> Site Location

The subject site is located on the northern side of Bagdad Street at property No. 32 (legally known as Lot B of DP361957), within the suburb of Regents Park. The site has a frontage of approximately 18 metres to Bagdad Street from the south. Refer to Figure 1 for a site locality map.

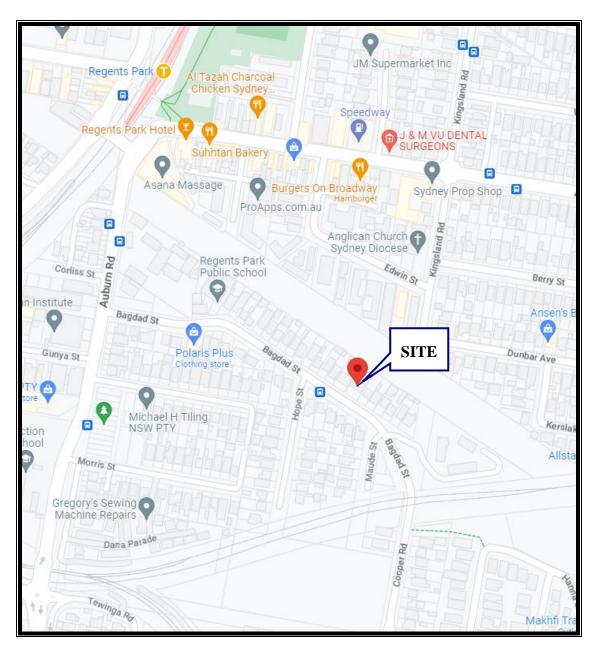


Figure 1: Site Locality Map



> Existing Site & Surrounding Land Use

The subject site has an area of 780.4m² and is currently occupied by a single residential dwelling. It is located in a mainly residential area, characterised by single dwellings, and is in close proximity to some commercial sites. The site is also located approximately 650 metres from Regents Park Railway Station, 1.9 km from Sefton Railway Station and 2 km from Berala Railway Station.



Photo 1: Site frontage to Bagdad Street



3 EXISTING TRAFFIC CONDITIONS

3.1 Road Network and Classification

Bagdad Street is a local road that runs in an east to west direction between its extension with Cooper Road (local road) to the east and Auburn Road (regional road) to the west. It intersects with Hope Street (local road) and Maude Street (local road) near the subject site.

3.2 Road Description and Traffic Control

Bagdad Street has a two-way undivided carriageway with a width between kerbs of approximately 10 metres. This carriageway generally provides one travel lane per direction, plus a kerbside parking lane on both sides of the road. At present, unrestricted parking is permitted on both sides of Bagdad Street, with the exception of the signposted 'No Parking 8.00am to 9.00am and 2.30pm to 3.30pm on School Days' along the frontage of the existing school to the west, and the signposted "No Stopping" towards its intersection with Auburn Road. The legal speed limit on Bagdad Street is signposted at 50km/h, with the exception of the signposted "School Zone" of 40km/h on school days. Bagdad Street intersects with Hope Street and Maude Street, which are both controlled by "T-priority", given to traffic travelling along Bagdad Street.



Figure 2: Aerial photograph of the subject site and surrounding road network



Photo 4: Bagdad Street at the subject site - facing east



Photo 5: Bagdad Street at the subject site – facing west



3.3 Current Traffic Flows

A traffic volume count was undertaken by Hemanote Consultants at the intersections of Bagdad Street / Auburn Road and Cooper Road / Moriarty Way in the vicinity of the subject site on Tuesday 02 May 2023, during morning period (7.00am to 10.00am) and afternoon period (3.00pm to 6.00pm), considering the childcare centre proposed hours of operation and traffic peak periods.

The traffic flows in the morning & afternoon peak hours are shown in Tables 1 and 2 below.

Traffic movement	Morning Peak Hour (Vehicles Per Hour)	Evening Peak Hour (Vehicles Per Hour)			
	7.30am – 8.30am	2.45pm – 3.45pm			
	Moriarty Way				
Westbound	83	25			
Eastbound	67	70			
	Cooper Road (North of Mo	oriarty Way)			
Northbound	158	85			
Southbound	147	143			
	Cooper Road (South of Mo	oriarty Way)			
Northbound	153	101			
Southbound	142	114			

Table 1: Current traffic flows in the vicinity of the subject site (on a typical weekday)



Traffic movement	Morning Peak Hour (Vehicles Per Hour)	Evening Peak Hour (Vehicles Per Hour)
	7.30am – 8.30am	2.45pm – 3.45pm
	Bagdad Street	
Westbound	299	194
Eastbound	137	226
	Auburn Road (North of Bagdad	Street) – 2 lanes
Northbound	689	483
Southbound	396	556
	Auburn Road (South of Bag	gdad Street)
Northbound	535	336
Southbound	297	377

Table 2: Current traffic flows in the vicinity of the subject site (on a typical weekday)

The results of the traffic volume counts undertaken determined that the traffic morning peak period on Bagdad Street / Auburn Road and Cooper Road / Moriarty Way were between 7.30am to 8.30am and the afternoon peak period was between 2.45 pm to 3.45pm on a typical weekday.

The existing traffic flows on Bagdad Street, Cooper Road, Moriarty Way and Auburn Road are appropriate for local roads and a regional road in a mainly residential area, where traffic is free flowing without major queuing or delays near the subject site in peak hours, with spare capacity.

It is determined that the existing mid-block level of service on both Moriarty Way and Cooper Street are at level 'A' in accordance with Table 4.4 of the Roads & Maritime Services' "Guide to Traffic Generating Developments - 2002" (shown on the following page). It is also determined that the existing mid-block level of service on Bagdad Street ranges between levels 'A' and 'B'.



The existing mid-block level of service on Auburn Road ranges between levels 'A' and 'C' to the south of Bagdad Street (1 lane in each direction) and to the north of Bagdad Street heading southbound (1 lane). The existing mid-block service on Auburn Road is at level 'A' to the north of Bagdad Street, heading northbound (2 lanes).

Level of Service	One Lane (veh/hr)	Two Lanes (veh/hr)
А	200	900
В	380	1400
С	600	1800
D	900	2200
E	1400	2800

Table 4.4: Urban road peak hour flows per direction RMS Guide)

> Current Intersection Performance

Average Vehicle Delay (AVD) and Level of Service (LOS) – The AVD and LOS provide a measure of the operational performance of an intersection, as indicated in Table 4.2 of the Roads & Maritime Services "Guide to Traffic Generating Developments - 2002" (shown below).

Level of Service	Average Delay per Vehicle (secs/veh)	Traffic Signals, Roundabout	Give Way & Stop Signs
А	< 14	Good operation	Good operation
В	15 to 28	Good with acceptable delays & spare capacity	Acceptable delays & spare capacity
С	29 to 42	Satisfactory	Satisfactory, but accident study required
D	43 to 56	Operating near capacity	Near capacity & accident study required
E	57 to 70	At capacity; at signals, incidents will cause excessive delays	At capacity, requires other control mode
		Roundabouts require other control mode	

Table 4.2: Level of Service Criteria for intersections (RMS Guide)

A SIDRA intersection performance analysis was undertaken for the existing intersections of Bagdad Street / Auburn Road and Cooper Road / Moriarty Way, in the vicinity of the subject site (Pre-development).

Refer to Figure 3 on the following page, showing the T-intersections layout controlled by T-priority traffic measures with associated 'Stop' signage at Bagdad Street / Auburn Road, and a roundabout at Cooper Road / Moriarty Way.



Bagdad Street, Cooper Road and Moriarty Way have undivided carriageways, with one through traffic lane in each direction. Auburn Road has an undivided carriageway, generally with one through lane in each direction, except to the east of Bagdad Street heading eastbound, which has two lanes.

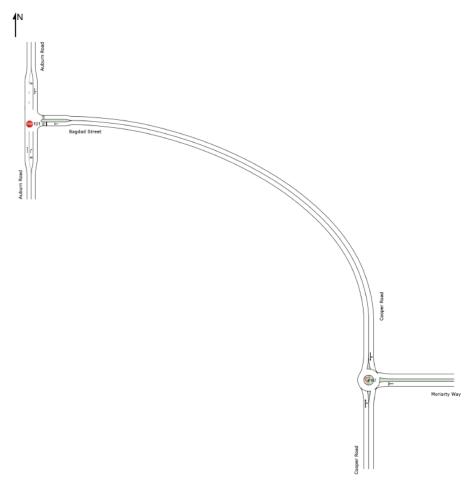


Figure 3: Existing Intersection Network Layout

The pre-development SIDRA performance analysis determined that the current operational performance of the existing T-intersection of Cooper Road / Moriarty Way is in good operation at a Level of Service (LOS) 'A'.

The existing T-intersection of Bagdad Street / Auburn Road during the AM & PM peak periods is at an overall Level of Service 'A', with the exception of the eastern approach on Bagdad Street, which is at an overall LOS 'B' during the AM and PM peak periods.

Refer to the summary of the results of the SIDRA intersection performance analysis attached in Appendix 'C' of this report.



3.4 Existing Transportation Services

The subject site has good access to public transport services in the form of trains and buses. The site is located approximately 650 metres from Regents Park Railway Station, 1.9 km from Sefton Railway Station and 2 km from Berala Railway Station.

Frequent bus services operate along Bagdad Street, Auburn Road, Cooper Road, Carlingford Street, Amy Street and Kingsland Road in close proximity to the subject site (i.e. bus routes N50, 908 and 909).

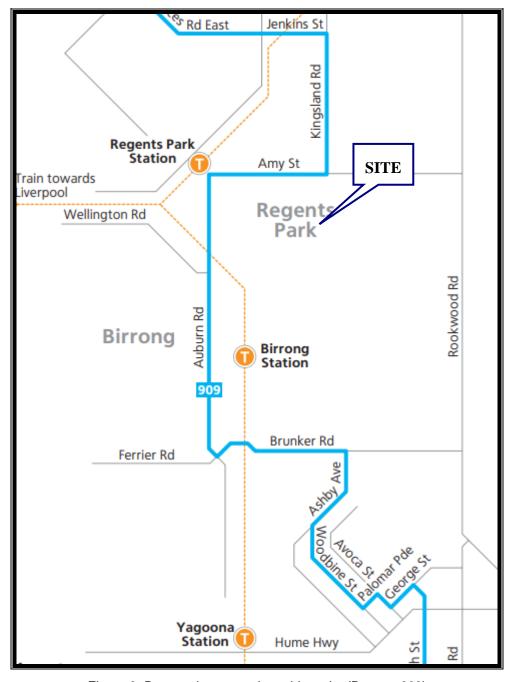
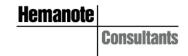


Figure 3: Bus services near the subject site (Bus no. 909)



4 PROPOSED DEVELOPMENT

4.1 Description of the proposal

The proposed development application seeks approval for the demolition of the existing residential dwelling located at **32 Bagdad Street**, **Regents Park**, and the construction of a childcare centre, accommodating up to 48 children places with basement parking.

The proposed development will include the following:

- Childcare centre accommodating up to forty-eight (48) children:
 - 12 children places between the ages of 0 to 2 years old (3 staff members);
 - 15 children places between the ages of 2 to 3 years old (3 staff members);
 - 21 children places between the ages of 3 to 6 years old (3 staff members).
- A total of thirteen (13) on-site car parking, with 7 car spaces allocated for staff parking and 6 visitor car parking spaces for the drop-off and pick-up of children (including 1 accessible parking space & an adjacent shared area), in addition to a turning area, in basement level. A minimum of three (3) bicycle storage spaces are to be provided on-site.
- There will be a maximum of nine (9) staff members on-site at any given time. The proposed hours of operation of the centre will be from 7.00am to 6.00pm on weekdays only.

Refer to *Appendix 'A'* for the proposed development plans.



4.2 Vehicular & Pedestrian Access

The vehicular access to and from the off-street parking facilities will be via a new access driveway crossing to be located in Bagdad Street. The access driveway is to have a width of 6.3 metres, which is adequate for a low volume (Category 1) access driveway in accordance with AS2890.1:2004 – Table 3.2.

The access driveway is to provide two-way vehicular movements, where two vehicles can pass each other at the same time without causing delays or congestion to traffic on the street. The proposed access driveway is located more than 6 metres from the tangent point of the adjacent kerbline, in accordance with Figure 3.1 of AS2890.1:2004.

Vehicular access is to be located and constructed in accordance with the requirements of AS2890.1:2004, where vehicles enter and exit the site in a forward direction at all times.

The existing vehicular crossing located in Bagdad Street is to be removed and replaced with new kerb, gutter and footpath, to be constructed to Council specifications, to restore on-street car parking spaces.

The clear sight line triangle (2.5m x 2m) between the driver's eye view and pedestrians is provided on the exit side of the driveway, as per Figure 3.3 of AS2890.1:2004. A separate pedestrian access gate is also provided at the front of the site to segregate pedestrians and vehicles and improve safety within the site.



4.3 On-site Parking Provision

Canterbury-Bankstown Development Control Plan 2023, Chapter 3.2, Section 2, requires on-site parking for childcare centres to be provided at a minimum rate of:

- 1 car space per 4 children.
- Bicycle 1 space per 4 staff.

Refer to Table 3 below for the required and proposed car parking provision for the subject development site:

Age Group	0-2 years	2-3 years	3-5 years	Total
Number of children	12	15	21	48
Staff to Children Ratio	1 to 4	1 to 5	1 to 10	-
Number of Staff	3	3	3	9
On-site parking required (12 car spaces)				
On-site parking proposed (7 spaces for staff & 6 spaces for parents/visitors)				
Compliance with on-site car parking				

Table 3: On-site parking requirement and provision

The proposed childcare centre for 48 children places would therefore require a minimum of 12 on-site car parking spaces and 3 bicycle storage spaces.

The proposed development provides a total of thirteen (13) on-site car parking, with 7 car spaces allocated for staff parking and 6 visitor car parking spaces for the drop-off and pick-up of children (including 1 accessible parking space & an adjacent shared area), in addition to a turning area, in basement level. A minimum of three (3) bicycle storage spaces are to be provided on-site.

Therefore, the proposed on-site parking provision is adequate for the proposed development and in compliance with Council's parking requirements.



4.4 On-site Parking Layout and Circulation

The layout of the on-site car parking area and manoeuvring arrangements has been designed to enhance vehicular and pedestrian access, where vehicles enter and exit the site in a forward direction, through the provision of adequate internal aisle width and turning space.

AS2890.1:2004 Parking facilities Part 1: Off-street car parking requires a minimum parking space width of 2.4 meters (for User Class 1A staff parking) and 2.6 metres (for User Class 3 short-term visitor parking) and a minimum length of 5.4 meters. The proposed off-street car spaces have a minimum width of 2.4 meters for staff and a width of 2.6 metres for visitors and a minimum length of 5.4 meters each, which is adequate.

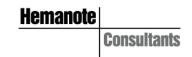
The accessible car parking space has a width of 2.4 metres, in addition to an adjacent 2.4 metres wide shared area, which is adequate in accordance with AS2890.6:2022.

Car parking spaces adjacent to walls or obstructions have been made wider than the minimum width, to accommodate full door opening in accordance with Clause 2.4.2(d) of AS2890.1:2004.

Clause 2.4.2 of AS2890.1:2004 requires a minimum aisle width of 5.8 metres for two-way aisles, adjacent to 90° angle parking. The proposed aisle has a minimum width of 6.6 metres, which is adequate for two-way traffic and manoeuvring into and out of parking spaces.

A **turning area** is provided towards the rear of the basement parking level, to allow vehicles to turn around and exit in a forward direction, if all other car parking spaces are occupied

The ramp to the basement level has a clear width of 5.5 metres, in addition to a 300mm kerb on either side and has a maximum grade of 1:20 (5%) for the first 6 metres within the site. It has a maximum grade of 1:4 (25%) with a change of grade of 1:8 (12.5%) over the 2 metres at either end of the ramp, which is adequate.



A minimum 2.2 metres headroom clearance is to be provided from the car park basement level to the underside of all services conduits and suspended stormwater pipelines, in accordance with Clause 5.3.1 of AS2890.1:2004. A "Maximum Headroom Clearance 2.2m" sign is to be erected at the entrance to the basement level and is to be clearly visible to all drivers. A minimum 2.5 meters headroom clearance is to be provided above the accessible parking space and its adjacent shared zone in accordance with Clause 2.4 of AS2890.6:2022.

A traffic convex mirror is to be installed at the bottom of the ramp (as shown on the basement plan), to provide drivers with further assistance with viewing oncoming traffic, as an additional safety and traffic management measure.

A dedicated pedestrian path has also been provided within the carpark, to provide a defined pedestrian path to the centre's access point. It is recommended that the onsite parking area be signposted with a speed limit of 10 km/h, to reduce speed environment, raise awareness of the presence of pedestrians and children and increase safety for all users of the carpark.

All vehicular manoeuvring within the site has been designed and checked using the B99 and B85 standard design vehicle turning paths from AS2890.1:2004 and Austroads. Refer to the vehicle swept paths diagrams attached in Appendix 'B' of this report.

Therefore, the car parking layout and vehicular circulation are adequate in accordance with AS2890.1:2004 and AS2890.6:2022, where vehicles are to enter and exit the site in a forward direction at all times.

4.5 Waste Collection & Deliveries

All waste storage is to take place within the dedicated garbage storage area located at-grade level. Waste Bins will be transported to the street kerbside for collection on waste collection day.



5 ON-STREET PARKING PROVISION

5.1 Existing Parking Controls

The subject site is located in a mainly residential area, where unrestricted parking is permitted on both sides of Bagdad Street, with the exception of the signposted 'No Parking 8.00am to 9.00am and 2.30pm to 3.30pm on School Days' along the frontage of the existing school to the west, and the signposted "No Stopping" towards its intersection with Auburn Road.

5.2 Impacts of Proposed Development on Parking

The parking demand resulting from the proposed childcare centre development can be accommodated within the proposed adequate and compliant on-site car and bicycle parking spaces for staff and visitors. The subject site has good access to existing public transport in the form of train and bus services.

Therefore, the proposed development will not have adverse impacts on parking in the surrounding area.



6 EXTERNAL TRAFFIC IMPACT

6.1 Estimated Future Traffic Generation

An indication of the potential traffic generation of the proposed development is provided by the RMS Guide to Traffic Generating Development - 2002.

The Guide specifies the following traffic generation rates for long-day care centres:

- 0.8 peak period vehicle trips per child between 7.00am and 9.00am; and
- 0.7 peak period vehicle trips per child between 4.00pm and 6.00pm.

Therefore, the proposed development with a total of <u>48 children places</u> has a total estimated traffic generation as follows:

- 39 morning peak period vehicle trips (20 In and 19 Out trips); and
- 34 afternoon peak period vehicle trips (17 In and 17 Out trips).

6.2 Projected Intersection Performance

Average Vehicle Delay (AVD) and Level of Service (LOS) – The AVD and LOS provides a measure of the operational performance of an intersection, as indicated in Table 4.2 of the Roads & Maritime Services "Guide to Traffic Generating Developments - 2002" (shown below).

Level of Service	Average Delay per Vehicle (secs/veh)	Traffic Signals, Roundabout	Give Way & Stop Signs
А	< 14	Good operation	Good operation
В	15 to 28	Good with acceptable delays & spare capacity	Acceptable delays & spare capacity
С	29 to 42	Satisfactory	Satisfactory, but accident study required
D	43 to 56	Operating near capacity	Near capacity & accident study required
E	57 to 70	At capacity; at signals, incidents will cause excessive delays	At capacity, requires other control mode
		Roundabouts require other control mode	

Table 4.2: Level of Service Criteria for intersections (RMS Guide)

A post-development SIDRA intersection performance modelling analysis was undertaken for the T-intersections of Bagdad Street / Auburn Road and Cooper Road / Moriarty Way in the vicinity of the subject site, and it was modelled as the proposed network layout as shown in Figure 5 on the following page.



Refer to the summary of the results of the SIDRA intersection performance analysis (undertaken for pre & post development, including the 10-year future growth) attached in Appendix 'C' of this report.

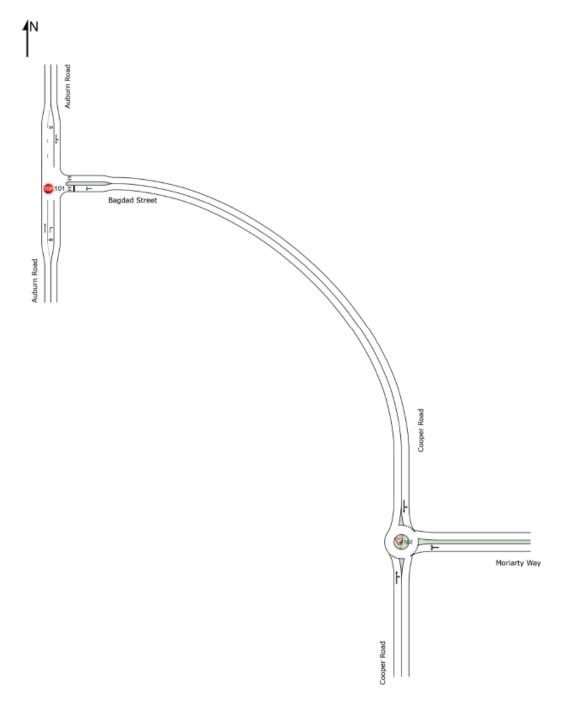


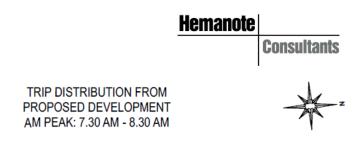
Figure 5: Intersection Network Layout



The following <u>assumptions</u> have been considered and adopted in the SIDRA Network Intersection modelling for the post-development conditions considering that the main access to and from the subject site is through Bagdad Street:

- Morning and afternoon peak traffic generation from RMS Guide to Traffic Generating Developments has been used, as outlined in Section 6.1 of this report.
- AM & PM Traffic generated by the development was equally adopted for inbound and outbound traffic.
- Pre-development network analysis is modelled for the base year (2023) and 10 years of future growth (2033) in surrounding traffic. The annual traffic growth rate for the 10-year future period was based on the SIDRA intersection analysis software, which allows for future analysis in surrounding traffic by applying a uniform growth rate of 2% for each year over the 10-year period.
- Post-development network analysis is modelled for Childcare Centre development is in operation and after 10 years of future growth in surrounding traffic.

These assumptions will result in the development trip distribution shown in Figures 6 and 7 for relevant traffic movement and modelled intersection.



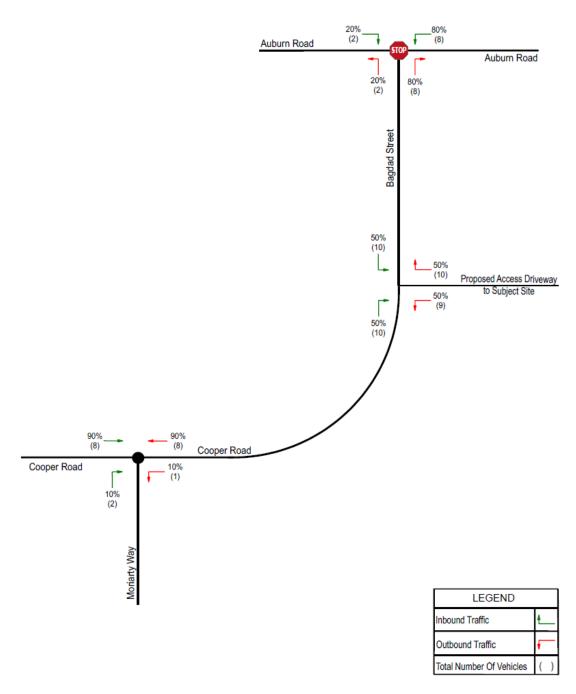
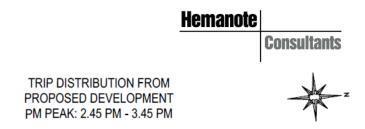


Figure 6: Development Traffic Distribution on the Surrounding Road Network - AM Peak



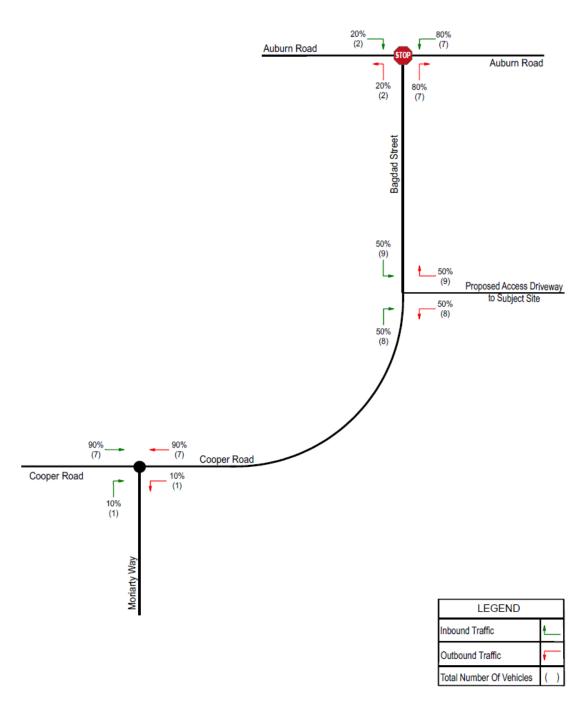


Figure 7: Development Traffic Distribution on the Surrounding Road Network - PM Peak



The outcome of the SIDRA modelling

A summary of the results of the SIDRA intersection performance analysis has been provided in Tables 4 to 7 below, as well as the SIDRA Movement Summary Tables attached in Appendix 'C' of this report.

Modelled Peak	Average LOS	Average Delay (sec)	DOS (Veh/C)
Base Year 2023 – Pre-Development	Α	7.4	0.810
Base Year 2023 – Post Development	Α	8.2	0.844
Future Year 2033 – Pre-Development	Е	72.4	1.292
Future Year 2033 – Post Development	E	86.0	1.350

<u>Table 4: Network SIDRA Modelling – Bagdad Street / Auburn Road – 7.30am – 8.30am</u>

Modelled Peak	Average LOS	Average Delay (sec)	DOS (Veh/C)
Base Year 2023 – Pre-Development	А	4.1	0.466
Base Year 2023 – Post Development	Α	4.3	0.469
Future Year 2033 – Pre-Development	Α	5.5	0.678
Future Year 2033 – Post Development	А	5.9	0.716

<u>Table 5: Network SIDRA Modelling – Bagdad Street / Auburn Road – 2.45pm – 3.45pm</u>

Modelled Peak	Average LOS	Average Delay (sec)	DOS (Veh/C)
Base Year 2023 – Pre-Development	Α	5.7	0.133
Base Year 2023 – Post Development	А	5.7	0.140
Future Year 2033 – Pre-Development	А	5.8	0.161
Future Year 2033 – Post Development	А	5.8	0.170

Table 6: Network SIDRA Modelling - Cooper Road / Moriarty Way - 7.30am - 8.30am

Modelled Peak	Average LOS	Average Delay (sec)	DOS (Veh/C)
Base Year 2023 – Pre-Development	А	5.1	0.111
Base Year 2023 – Post Development	А	5.1	0.117
Future Year 2033 – Pre-Development	А	5.2	0.134
Future Year 2033 – Post Development	Α	5.2	0.141

<u>Table 7: Network SIDRA Modelling – Cooper Road / Moriarty Way – 2.45pm – 3.45pm</u>

It can be concluded from the results of the SIDRA modelling that:

Base Year

- The current operational performance (pre-development) at the T-intersection of Cooper Road / Moriarty Way is currently operating at a Level of Service (LOS) 'A' during weekday AM & PM peak.
- The current operational performance (pre-development) at the T-intersection of Bagdad Street / Auburn Road during the AM & PM peak periods is at an overall Level of Service 'A', with the exception of the eastern approach on Bagdad Street, which is at an overall LOS 'B' during the AM and PM peak periods.
- The proposed childcare centre (post-development) will not alter the current LOS
 at the subject intersections and will continue to operate at its current levels of
 service during weekday AM & PM peak.



Future Year

- The future 10-year (pre-development) analysis indicates that the subject T-intersection of Cooper Road / Moriarty Way will operate at LOS 'A', during weekday AM & PM peak.
- The future 10-year (pre-development) analysis indicates that the subject T-intersection of Bagdad Street / Auburn Road will continue to operate at an overall Level of Service 'A' during the PM peak period, with the exception of the eastern approach on Bagdad Street, which will operate at an overall LOS 'B'. The AM peak period, however, will continue to operate at an overall Level of Service 'A', with the exception of the eastern approach on Bagdad Street, which will operate at an overall LOS 'F'.
- The proposed childcare centre (post-development) will <u>not</u> alter the future LOS at the subject intersections and will continue to operate at the predicted future levels of service.

Therefore, the estimated traffic generation from the proposed development is of low impact on existing flows on Bagdad Street and surrounding streets and will not have adverse impacts on the current operational performance of the subject existing intersections, which will continue to operate at the same levels of service.



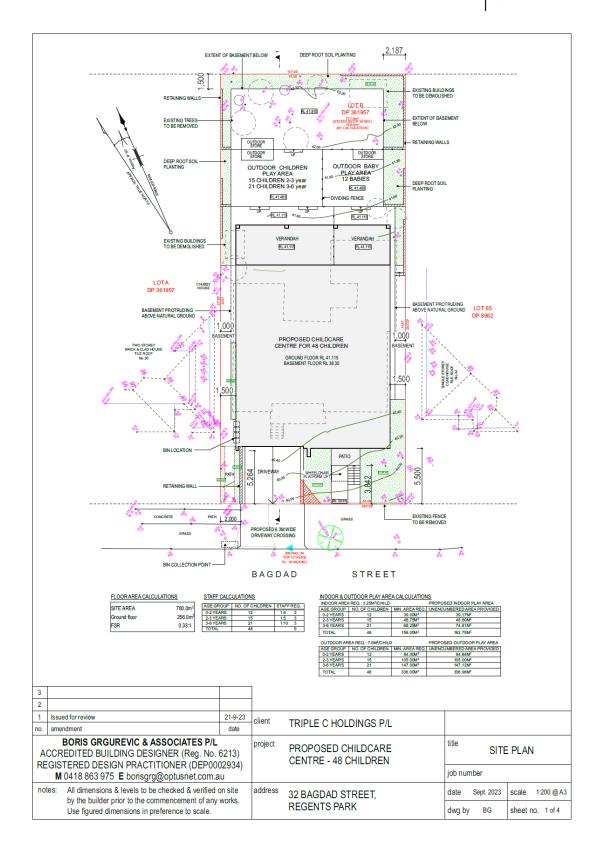
7 CONCLUSION

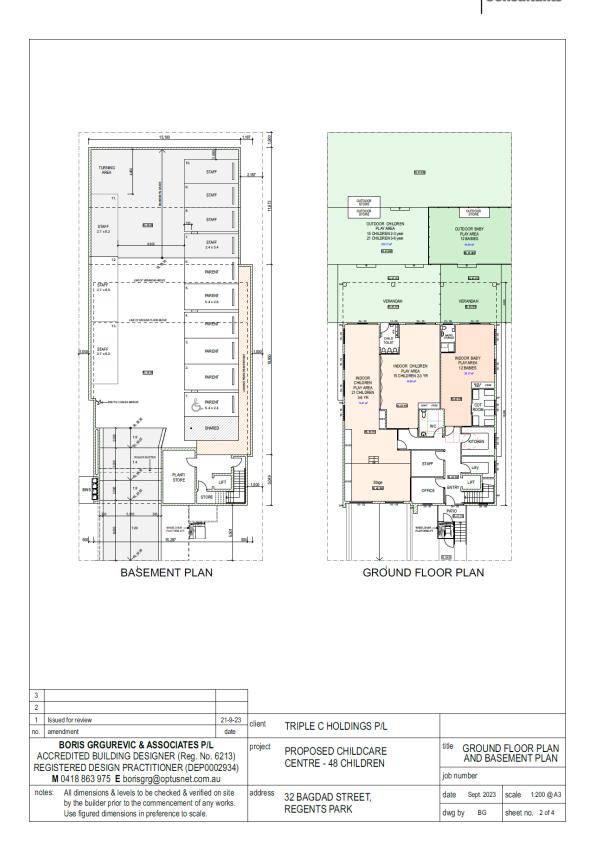
It can be concluded from the traffic and parking impact assessment that the proposed childcare centre development at **32 Bagdad Street**, **Regents Park**, will not have adverse impacts on existing traffic or parking conditions and is worthy of Council's support in its current form.

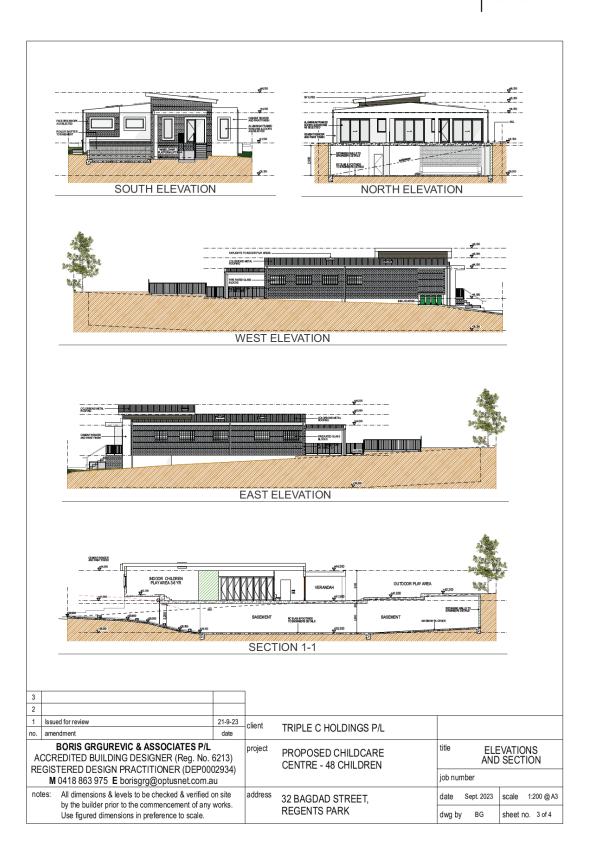
- The current traffic flows on Bagdad Street, Cooper Road, Moriarty Way and Auburn Road are appropriate for local roads and a regional road in a mainly residential area, where traffic is free flowing without major queuing or delays in peak hours, with spare capacity.
- The estimated traffic generation from the proposed development is of low impact on existing flows on Bagdad Street and surrounding streets and will not have adverse impacts on the current operational performance of the subject existing intersections, which will continue to operate at the same levels of service. The traffic generated by the proposed childcare centre development can be readily accommodated within the existing road network.
- The potential increase in the number of vehicle movements in and about Bagdad Street and adjacent streets will not have adverse impacts on the amenity of the area.
- The parking demand resulting from the proposed childcare centre can be easily accommodated within the proposed adequate and compliant off-street car and bicycle parking for both staff and visitors/parents, which is in compliance Council's parking requirements.
- The on-site vehicular access, car parking layout and vehicular circulation is adequate for the proposed development and in accordance with AS2890.1:2004 and AS2890.6:2022, where vehicles are to enter and exit the site in a forward direction at all times.
- The subject site has good access to existing public transport services.
- The proposed development will not have adverse impact on parking in the surrounding area.



Appendix A – Proposed Development Plans

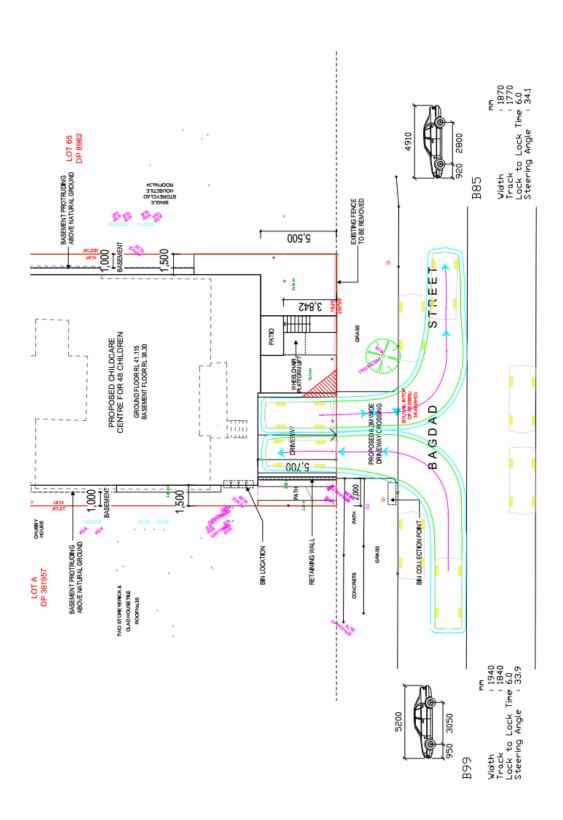




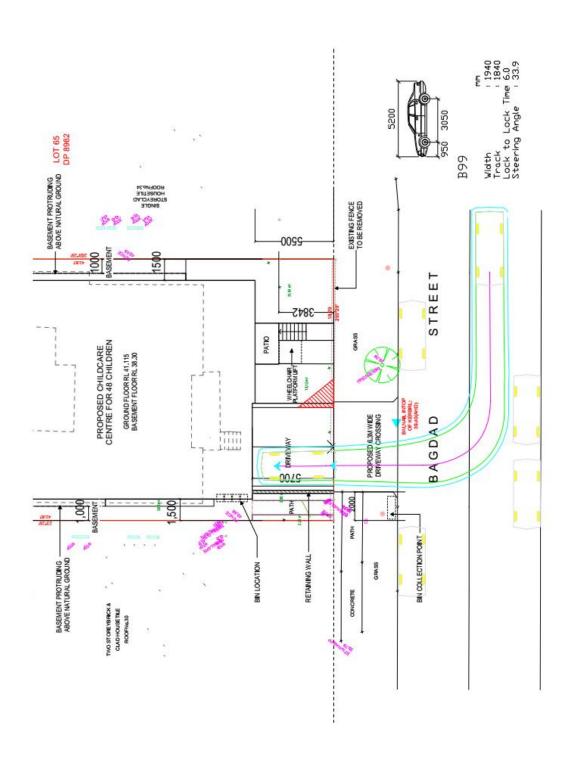




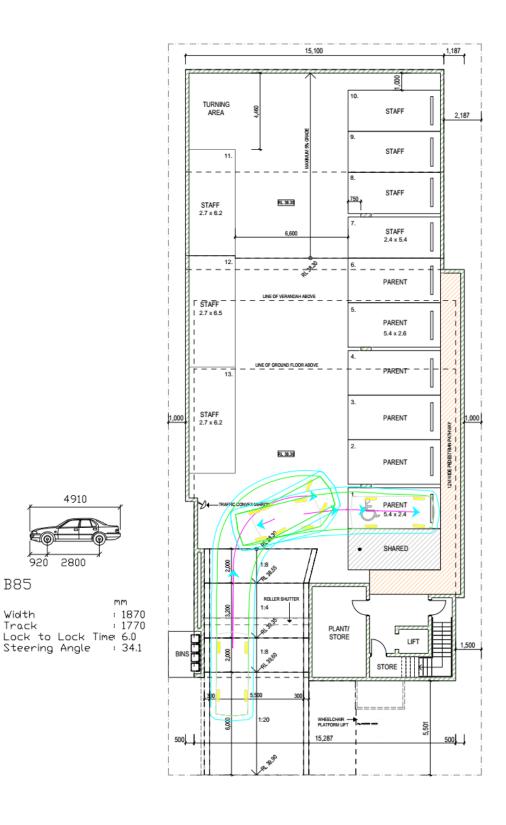
Appendix B - Vehicle Swept Paths

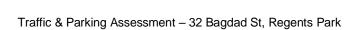


Hemanote Consultants



Hemanote Consultants



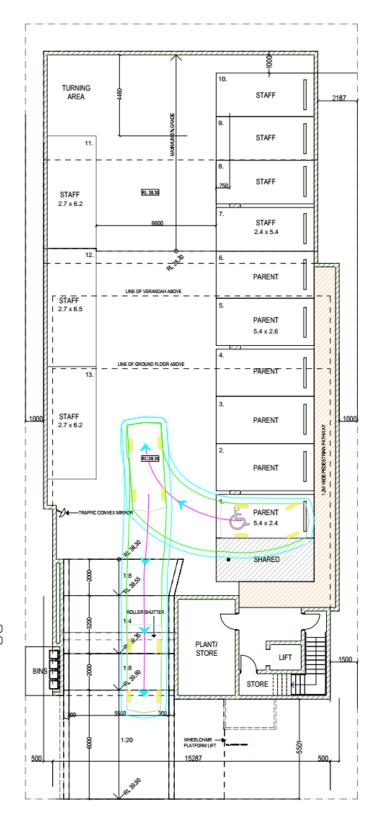


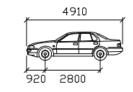
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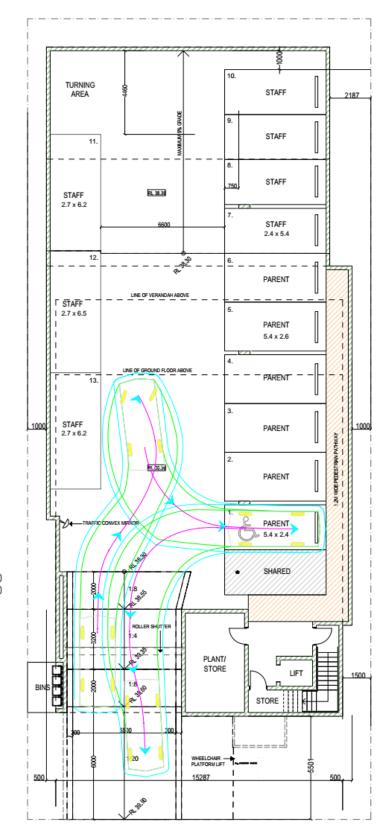
Width Track

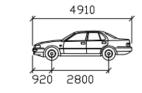




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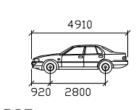




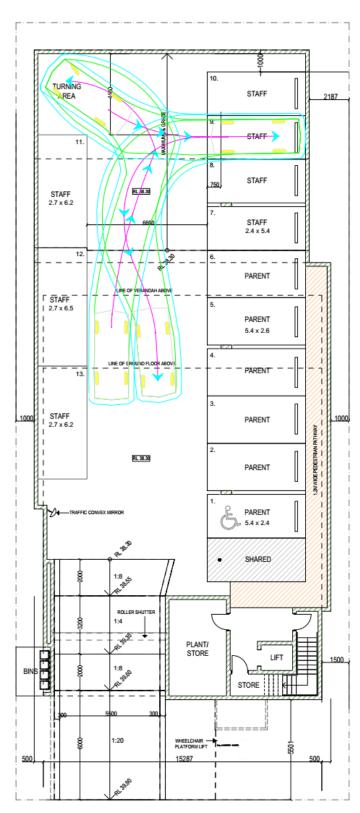
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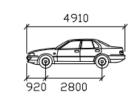
Width : 1870 Track : 1770 Lock to Lock Time 6.0 Steering Angle : 34.1

mm

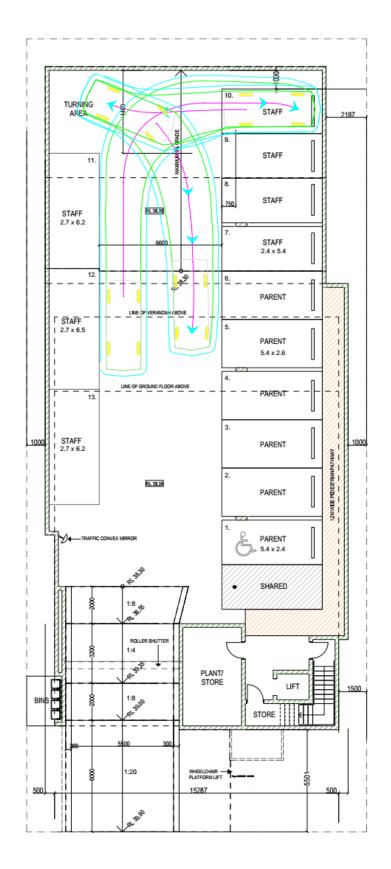


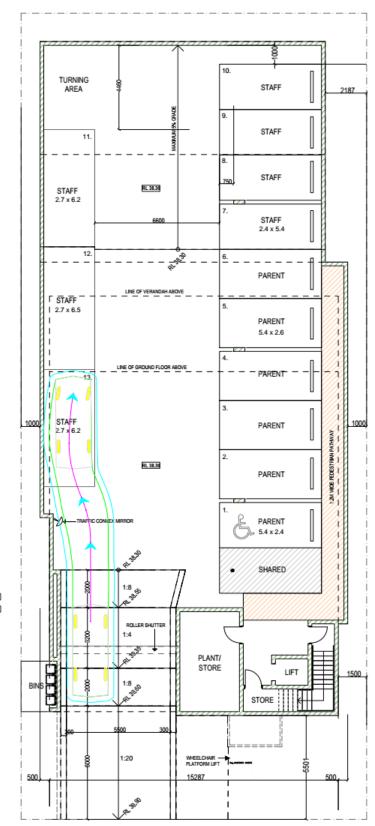
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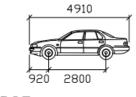




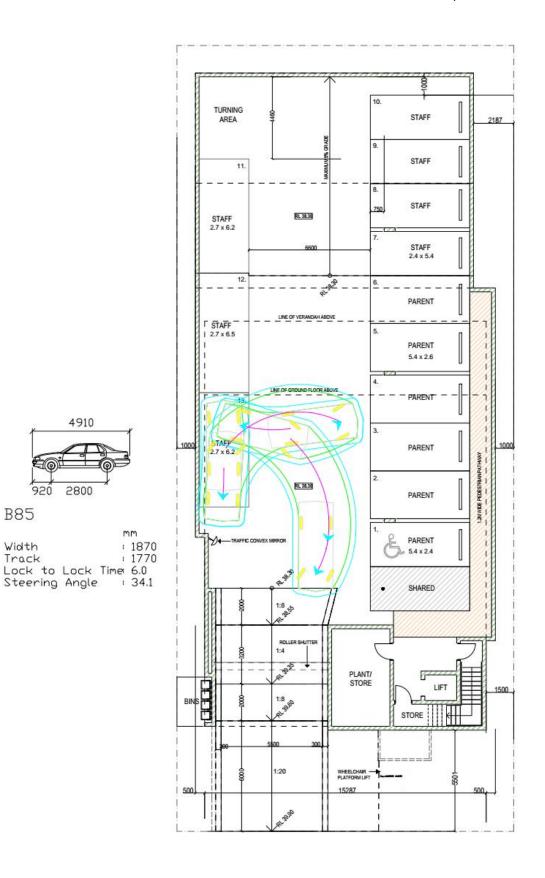
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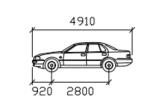






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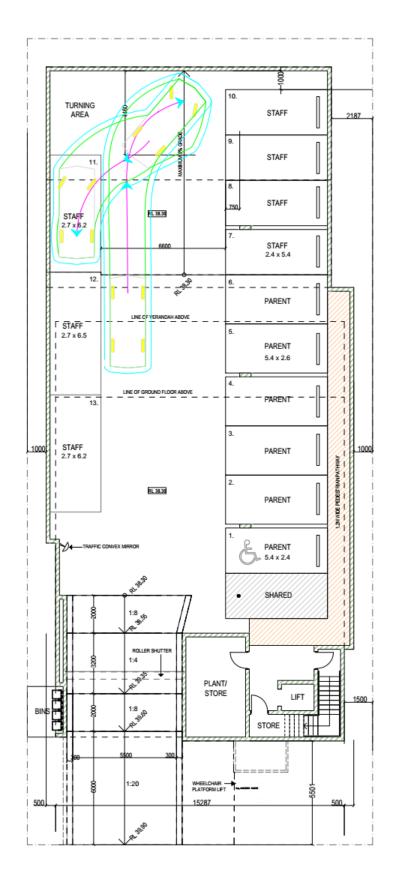


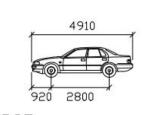


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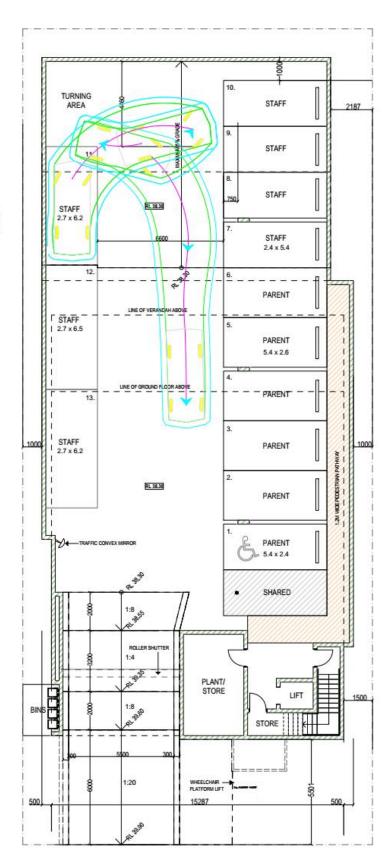
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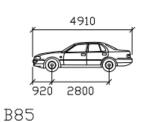
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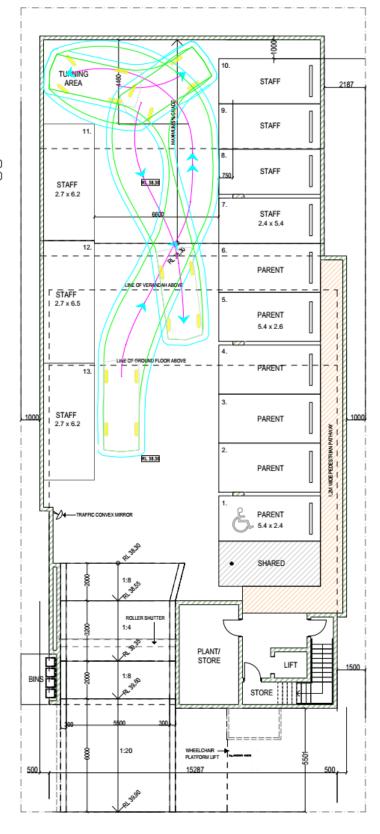




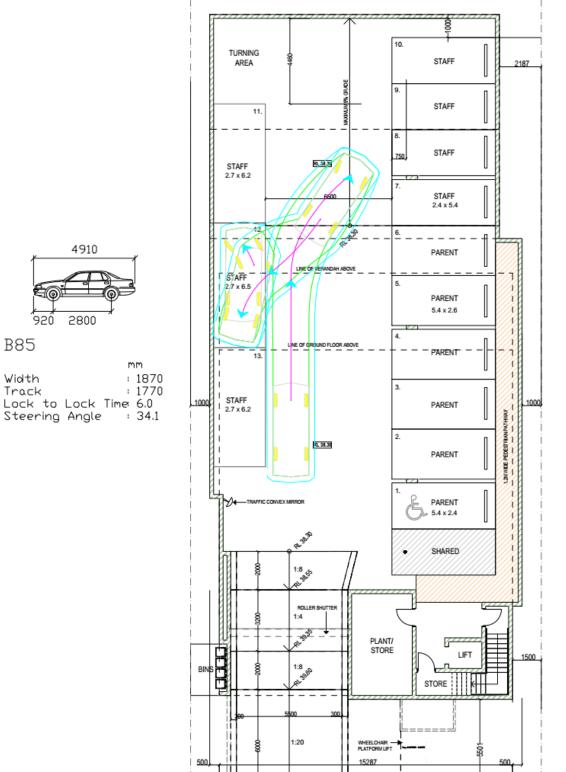
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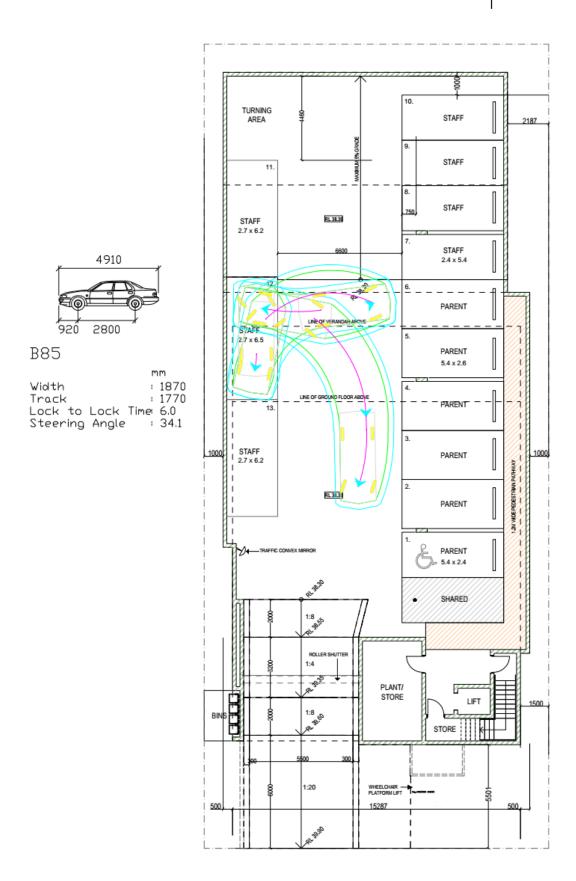






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Width





Appendix C – SIDRA Intersection Analysis

MOVEMENT SUMMARY - 7.30am - 8.30am - Base Year 2023 - Bagdad Street / Auburn Road

			ቯ	Pre-Development	e ve	obu	ent					-				т.	Post-Development	-De	/elo	pme	ţ				
Vehicle Move Mov Tum ID	cie Movement Performance Turn DEMAND ARBIN FLOMS FLOM [Total HV] [Total wehh % wehh	ARR C PEC V Total	Z 2 Z Z	26 A	Aver Level of Delay Service sec		AVERAGE BACK OF QUEUE [Veh. Det]	¥ —	Section Sop	eAver No P Cycles	Speed Th	Vehic IID	cle Move Turn	fehicle Movement Performance flow Turn DEMAND ARRIV TOWNS FLOW [Total HV] [Total Hv] vehh % vehh	Performal NS AR NS ITE NS in Properties	MARNAL RLOWS (Total HV)	8 8 ₹	Aver Delay	Level of Service	AVERA OF O	AVERAGE BACK OF QUEUE (Veh. Dist] veh. m	e e	Effective A Step Rate	Cydes Cydes	Speed In
South: Aubum Road	Road											South	Aubi	Road											
	537 0	0.0 537	0.0 0.277		0.0 LOSA			0000	0000 0	0.00	898	2	F	537 0	0.0 537	0.0 7	0.278	0.0	LOSA	000	0.0	000	000	000	59.8
	26 0	0.0 26	00	57	52 LOSA	A 00	0 0.3	0.45	9 0.60	0 0.45	246	е	23	35 0	0.0 35	0.0	0.029	5.2	LOSA	0.0	0.3	0.45	0.61	0.45	24.5
Approach	983 0		000			NA O		000	2 0.03	3 0.02	288	Approach		572 0	0.0 572	2 0.0	0.278	0.3	M	0.0	0.3	0.03	0.04	0.03	58.5
East Bagdad Street	Sheet											East	Bagdad Street	Street											
	14 0.	0.0 14	00	0.810 18	18.9 LOSB			68.0	9 1.38	8 230	36.5	4	2	16 0	0.0	0.0	0.844	212	10SB	27	18.9	0.90	1,45	2.59	36.3
22 9	301 0	0.0 301	000	0.810 28	28.4 LOSB	B 24	4 16.5	68.0	9 138	8 230	38.9	9	R2	309 0	0.0 309	0.0 6	0.844	30.9	D SO1	2.7	18.9	0.90	1.45	2.59	37.8
	315 0	0.0 315	00	0.810 28	28.0 LOSB		16.5	680 9	9 1.38	8 2.30	38.8	Approach		325 0	0.0 325	2 0.0	0.844	30.5	DSOT	27	18.9	06:0	1.45	2.59	37.6
North: Aubum Road	Road											North	North: Aubum Road	Road											
7 12	118 0.	118	000		5.6 LOSA			000	0 0.17	7 0.00	625	1	27	120 0	0.0 120	00 0	0.218	9.6	LOSA	0.0	0.0	0000	0.17	000	52.8
		0.0 299	00	0.217 0	0.0 LOSA	A 0.0	0.0	000	0 0.17	000 4	542	60	F	299 0	0.0 299	0.0 6	0.218	0.0	LOSA	0.0	0.0	0.00	0.17	000	22
proach	417 0.	0.0 417	00	0.217	1.6 N	NA 0.0	0.0	000	0 0.17	0.00	53.9	Approach		419 0	0.0 419	0.0 6	0.218	1.6	NA.	0.0	0.0	0.00	0.17	0.00	53.8
Al Vehicles	1295 0	00 1296	1295 00 08	0.810 7	7.4 N	NA 24	4 16.5	5 023	3 0.40	75.0 0	45.5	All Ve	All Vehicles	1316 0	0.0 131	1316 0.0	0.844	8.2	NA	2.7	18.9	0.23	0.43	990	44.4

MOVEMENT SUMMARY - 7.30am - 8.30am - Base Year 2023 - Cooper Road / Moriarty Way

				Pre	-Ç	velc	Pre-Development	ent									Po	st-D	eve	lop	Post-Development				
Vehicle M	icie Movement Performance	I Perfo	rmance										Vehicle	Moveme	cle Movement Performance	rmance									
Mov Tum ID	图片图章	DEMAND FLOWS [Total HV]	ARRIVAL FLOWS [Total HV]	Strate N		Aver. Level of Delay Service sec		AVERAGE BACK OF QUEUE [Yeh: Dist] veh: m	§9	a. Effective Aver. No. Aver. e. Step Cycles Speed Rate. kmh	Cydes Cydes	Aver. Speed km/h	Mov Tum ID	H도 결혼	DEMAND FLOWS [Total HV]	ARRINAL FLOWS [TotalHV]			Aver Level of Jelay Service sec		AVERAGE BACK OF QUEUE [Veh Dist] veh m	80	19 B	p. Effective-Aver No. Aver us. Stop. Cycles Speed Rate. Knith	to. Aver. es Speed terrifi
South: Cooper Road	per Road	_											South: Cooper Road	oper Ros	pe										
2 TI	127	000	127 0.0			L08A		20	0.20	0.52	070	39.8	2 T	136	0.0	136 0	0.0 0.140		5.1 LOS	4	0.3 2.1	1 021		0.52 0.21	21 39.8
3 82	×	0.0	34 0.0	0 0.133	3 8.0	LOSA	0.3	20	0.20	0.52	020	39.0	3 R2	2 34	0.0	34 0	0.0 0.140		8.0 LOS	4	0.3 2.1	1 021		0.52 0.21	21 39.0
Approach	161	000	161 0.0		5.7	LOSA		20	0.20	0.52	020	39.5	Approach	169	0.0	169 0	0.0 0.140		57 108	4	0.3 2.	1 021		0.52 0.3	0.21 39.
East. Monarty Way	irty Way												East Moriarty	iarty Way	22										
4 12	33	0.0	32 0.0			3 LOSA		1.1	0.29	1970	0.29	38.7	4 12	32	0.0	32 0	0.0 0.084		5.4 LOS	4					0.30 38.6
6 R2		0.0	96 000	0 0.082	2 8.2	2 LOSA	0.2	17	0.29	1970	0.29	27.1	9	Sint.	0.0	58 0	0.0 0.084		8.2 LOS	4	0.2 1.1	1 0.30		0.61 0.3	0.30 27.0
Approach	87	0.0	87 0.0	0 0.082	1.7	LOSA		17	0.29	1970	0.29	33.2	Approach	88	0.0	89	0.0 0.084		7.2 LOS	4	0.2 1.1	1 0.30		0.61 0.	0.30 33.0
North: Cooper Road	per Road												North: Cooper Road	oper Roa	p										
7 12	37	000	37 0.0	0 0.120	5.1	L08A		17	0.14	0.49	0.14	46.4	7 12	38	0.0	38	0.0 0.127		5.1 LOS	4	0.3 1.9	9 0.15		0.49 0.	0.15 46.4
8	118	00	118 0.0	0 0.120	0 20	LOSA	0.2	1.7	0.14	0.49	0.14	532	8 T1	126	0.0	126 0	0.0 0.127		5.0 LOS	4	0.3 1.9	9 0.15		0.49 0.	0.15 53.2
Approach	78	0.0	155 0.0	0 0.120	0.5	LOSA		1.7	0.14	0.49	0.14	51.6	od	16	0.0	164 0	0.0 0.127		5.0 LOS	A	0.3 1.9	9 0.15		0.49 0.	0.15 51.6
All Vehicles	3 403	000	403 0.0	0 0.133	3 5.7	7 LOSA	0.3	20	070	0.53	070	46.6	All Vehicles	es 423	0.0	423	0.0 0.140		5.7 LOSA		0.3 2.1	1 0.20		0.53 0.3	0.20 46.7

MOVEMENT SUMMARY - 2.45pm - 3.45pm - Base Year 2023 - Bagdad Street / Auburn Road

Vehicle Movement Performance Mov Turn DEMAND ARRWAL ID FINISH HV Nost HV South Authum Road 2 T1 327 0.0 327 0.0 0.169 3 R2 26 0.0 26 0.0 0.027 Approach 354 0.0 354 0.0 0.169 East Bagdod Street 4 L2 23 0.0 23 0.0 0.446 6 R2 181 0.0 181 0.0 0.446 Modif. Authum Bood Modif. Authum Bood	Aver Level of Debay Senton sec 0.0 LOSA 6.1 LOSA 0.5 NA	AVERAGE BACK OF CURJE (Veh. Det)	App age		1													
Mft. Authum Road T1 327 0.0 327 0.0 0.169 P2 28 0.0 26 0.0 0.027 proach 354 0.0 354 0.0 0.169 sst. Bagdad Sheet L2 23 0.0 23 0.0 0.446 P2 181 0.0 181 0.0 0.446 proach 204 0.0 204 0.0 0.446 Mft. Authum Boad	The second second second	00 00	l	Effective.No Stop Cycles Rate	S Speed	Vehicle M Mov Tum ID	Overmen DEN P.C.	Movement Performance n DEMAND ARRIV FLOWS FLOW Total HV Total HV Webfil % webfil	mance ARRIVAL R.OWS [Total HV]	S. S	Aver. Delay	Level of Service	1000	AVERAGE BACK OF QUEUE (Veh. Dist.)	Open Place	Electric Associates As	Aver No Cycles	Speed finh
T1 327 0.0 327 0.0 0.169 P2 26 0.0 26 0.0 0.027 proach 354 0.0 354 0.0 0.169 st Bagdad Shreet L2 23 0.0 23 0.0 0.446 P2 181 0.0 181 0.0 0.446 proach 204 0.0 204 0.0 0.446						South: Aubum Road	um Road								-			
Proceeds 354 00 354 00 0.055 proceeds 354 00 354 00 0.169 proceeds 23 00 23 00 0.446 proceeds 204 00 204 00 0.446 proceeds 204 00 204 00 0.446			000	000 000	593	2 T1	327	000	327 0.0	0 0.169	0 9	0 LOSA	0.0	0.0	0.00	000	0000	59.9
st Bagdad Sheet 12 23 00 23 00 0446 R2 181 00 181 00 0446 proach 204 00 204 00 0446		1		11004		Approach	10000	0.0		100	0	2	0.1	0.4	0.05	900	900	57.5
12 23 00 23 00 046 R2 181 00 181 00 0446 groach 204 00 204 00 0446 Mt. Jahlum Boost						East Bagdad	ad Street											
R2 181 0.0 181 0.0 0.446 proach 204 0.0 204 0.0 0.446 oths Authum Board	11.6 LOSA		0.70	108 1.00	0 429	4 12	K	0.0	25 0.0	0 0.469	11.8		0.8	5.9	0.71	109	104	42.7
proach 204 0.0 204 0.0 0.445	17.1 LOSB	0.8 5.5	0.70	1.08 1.00	0 45.0		88	0.0	188 0.0	0 0.469	17.5	8 COS B	0.8	6.9	0.71	1.09	1.02	44.8
Jodh Juhum Board	16.5 LOSB	0.8 5.5	0.70	1.08 1.00	0 44.8	Approach	214	0.0	214 0.0	0 0.469	16	8 LOSB	0.8	6.9	0.71	1.09	104	44.5
Collect Automit Folia						North: Aubum Road	um Road											
. L2 212 0.0 212 0.0 0.306	5.6 LOSA	0.0 0.0	0000	0.00 0.00	0 512	7 12		0.0	214 0.0		5	S LOSA	0.0	0.0	000	0.22	0.00	5
8 T1 374 0.0 374 0.0 0.306	0.0 LOSA	0.0 0.0	0000	0.21 0.00	123 0	8	374	0.0	374 0.0	0.307	0	0 LOSA	0.0	0.0	000	0.22	000	52.7
Approach 585 0.0 585 0.0 0.306	2.0 NA	00 00	000	021 0.00	0 522	Approach	587	0.0	587 0.0	0 0.307	2.0	NA O	0.0	0.0	0.00	0.22	000	522
All Vehicles 1143 0.0 1143 0.0 0.446	4.1 NA	0.8 5.5	0.14	0.32 0.19	9 49.6	All Vehicles	3 1162	0.0	1162 0.0	0 0.469	43	N N	0.8	5.9	0.15	0.33	0.21	49.3

MOVEMENT SUMMARY - 2.45pm - 3.45pm - Base Year 2023 - Cooper Road / Moriarty Way

				Pre	-Pe	velc	Pre-Development	ent										ost	Ģ	velo	Post-Development	'n				
Vehicle Movement Performance Mov Turn DEMAND APRIV ID FLOWS FLOW I Total N. Total vehit % vehit	Vernent Perf DEMAND FLOWS [Total HV]	AND A	Mance ARRIVAL FLOWS (Total HV)	25 =	Aver Delay	Aver Level of Delay Service 990	≨_57*	BAGE BACK OF QUEUE eh. Dist]	g g	a Effective-Aver. No Aver e Stop Cycles Speed Rate	Oydes:	Aver	Vehic Mov	cie Move Tum	Movement Performance n DEMAND ARRIVA LLOWS FLOW [Total HV] [Total HV with % with	AR C	Mance ARRIVAL FLOWS Total HV]	g.ll.	Aver Delay Sec	Level of Service	AVERA O OF O	AVERAGE BACK OF QUEUE Veh Dist]	8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8	Effective Ave Stop C Rate	S SB	Speed Amer
Souffi: Cooper Road	Road												South	South: Cooper Road	Road											
2 TI		0.0		0.000	4.8		0.1	0.1	0.03	0.54	0.03	415	2					9200	4.8	LOSA	0.2	=	900	0.53	0.04	41.5
3 82			20 0.0		11			10	0.03	0.54	0.03	40.2	m				0.0	9.000	7.7	LOSA	07	=	0.04	0.53	0.04	402
Approach	106		106 0.0	0.000	5.4	LOSA		1.0	0.03	25	0.03	41.2	Approach		114 0	0.0 114	000	0.076	5.3	LOSA	0.2	17	0.04	0.53	0.04	412
East Monarty Way	Way												East	East Monarty Way	Vay											
4 12	23	0.0	23 0.0	0 0024	5.1	LOSA	0	0.3	0.25	0.54	0.25	40.8	4		23 0	10 23	00	0.025	5.2	LOSA	0.0	0.3	0.26	0.55	0.26	40.6
6 R2			3 0.0		8.0	LOSA	0.0	0.3	0.25	0.54	0.25	29.5	9	R2	4 0	0.0 4	0.0	0.025	8.0	LOSA	0.0	0.3	0.26	0.55	0.26	292
Approach	36	0.0	26 0.0	0 0024	5.5	LOSA	0	0.3	0.25	0.54	0.25	40.1	Approach		27 0	0.0 27	0.0	0.025	5.6	LOSA	0.0	0.3	0.26	0.55	0.26	39.7
North: Cooper Road	Road												North	Cooper Road	Road											
7 12	35	0.0	54 0.0	0 0111	5.0	LOSA		1.5	0.10	050	0.10	46.5	7		98	0.0 55	0.0	0.117	5.0	LOSA	0.2	1.6	0.10	0.50	0.10	46.5
8 TI			97 0.0		4.9		0.2	1.5	0.10	0.50	0,10	53.4	60	I	104 0	0.0 104	0.0	0.117	4.9	LOSA	0.2	1.6	0.10	0.50	0.10	53.4
Approach	151	0.0	151 0.0	0.111	4.9	LOSA		12	0.10	050	0.10	609	Approach		159 0	0.0 159	0.0	0.117	4.9	LOSA	0.2	1.6	0.10	0.50	0.10	51.0
All Vehicles	283	0.0 2	283 0.0	0.111	5.1	LOSA	0.2	1.5	60'0	0.52	60'0	48.6	All Vehicles		300 0	0.0 300	0.00	0.117	5.1	LOSA	0.2	1.6	6000	0.51	0.09	48.6

MOVEMENT SUMMARY - 7.30am - 8.30am - Future Year 2033 - Bagdad Street / Auburn Road

MAND AGENAL Dag OWS FLOWS Sam H/1 Tedat H/1 % weth % we 0.0 644 0.0 0.033 0.0 32 0.0 0.029 0.0 676 0.0 0.333 0.0 16 0.0 1.292 0.0 361 0.0 1.292 0.0 361 0.0 1.292 0.0 378 0.0 1.292	Lewel of Service													-					
Overnert Performance DEMAN ARRWAL FLOWS FLOWS FLOWS FLOWS Salm Verbin W Total W Verbin W Total W Verbin W Verbi																			
DEMAND ARRANI Day Perons	_		3		- CA		Vehici	icle Mover	nent Per	Movement Performance					The same of the sa	8	3	8	-
um Road 644 00 644 00 0333 32 00 32 00 0029 676 00 676 00 0333 ad Street 16 00 16 00 1292 2 361 00 361 00 1292 2 378 00 378 00 1292 2	IOSA	AVERAGE BACK OF QUEUE [Veh. Dist] veh. m		Prop. EffectiveA Que Stop Rate	reAver.No op Cycles	Aver. Speed	Mov		DBMAND FLOWS [Total HV]	ARRIVAL FLOWS 7] [Total HV]		Deg Aver Sam Delay vic sec	wer. Level of elay Service sec		AVERAGE BACK OF QUEUE [Veh Det] veh m	g d	Effective Stop Rate	Awer. No.	Speed km/h
644 0.0 644 0.0 0.333 32 0.0 0.029 676 0.0 676 0.0 0.333 and Street 6 0.0 16 0.0 1292 23 361 0.0 361 0.0 1292 23 378 0.0 378 0.0 1292 22 22 378 0.0 378 0.0 1292 22							South	South: Aubum Road	peo										
32 00 32 00 0029 676 00 676 00 0333 ad Street 16 00 16 00 1292 23 361 00 361 00 1292 22 378 00 378 00 1292 22				000 000	000 000	59.8	2	11 6	644 0.0	3	0.0 0.332		0.0 LOSA		0.0 0	000	0000	0.00	59.8
676 0.0 676 0.0 0.333 ad Sweet 16 0.0 16 0.0 1.292 22 361 0.0 361 0.0 1.292 22 378 0.0 378 0.0 1.292 22	5 LOSA	0.0	03 0	0.49 0.64	64 0.49	23.4	m		42 0.0	42	0.0 0.039		5.7 LOSA	A 0.1		0.50	0.65	0.50	23.4
st Bagdad Shreet 1.2 16 0.0 16 0.0 1292 R2 361 0.0 378 0.0 1292 proach 378 0.0 378 0.0 1292	NA NA			0.02 0.03	33 0.02		Approach		989 0.0	989	0.0 0.332		0.4 N	NA 0.1	1 0.4	0.03	0.04	0.03	58.4
L2 16 0.0 16 0.0 1292 R2 361 0.0 361 0.0 1292 proach 378 0.0 378 0.0 1292							East B	Bagdad Street	reet										
R2 361 0.0 361 0.0 1292 proach 378 0.0 378 0.0 1292			171.3	1.00 4.0	4.55 14.80	7.5	7		19 0.0	19	0.0	350 334.7	17 LOSF	F" 28	4 198.5	1.00	4.96	16.36	9.9
proach 378 0.0 378 0.0 1292	LOSE	24.5	1713	1.00 4.9	4.55 14.80	8.7	9	R2 3	371 0.0	37.1	0.0 1.3	1,350 345.6		F" 28.	(4 198.5	1.00	4.96	16.36	7.6
Months Address David			1713	1.00 4.5	4.55 14.80	8.7	Approach		390 0.0	390	0.0 1.3	350 345.1	11 LOSF	333	28.4 198.5	1.00	4.96	16.36	7.6
NOTELL PUDDITT PUDDI							North	North: Aubum Road	peo										
141 0.0 141 0.0 0.260	LOSA		0.0	71.0 0.00	17 0.00	52.9	7	1 7	144 0.0	144	0.0 0.262		5.6 LOSA	A 0.0	0.0 0	000	0.17	0.00	52.8
8 T1 359 0.0 359 0.0 0.260 0.0	LOSA	0.0	0.0	0.00 0.0	0.17 0.00	1.12	60		369 0,0	369	0.0 0.262		0.0 LOSA		0.0 0.0	00'0	0.17	0.00	$\frac{2}{2}$
proach 500 0.0 500 0.0	N.		0.0	0.00	0.17 0.00	53.8	Approach		503 0.0	203	0.0 0.262		1.6 N	NA 0	0.0 0.0	000	0.17	0.00	53.8
All Vehicles 1554 0.0 1554 0.0 1292 724	N	24.5	1713 (0.25	1.17 3.61	14.3	All Vehicles		1579 0.0	1579	0.0 1.3	350 86	96.0 N	NA 28	4 198.5	0.26	130	4.06	12.6

MOVEMENT SUMMARY - 7.30am - 8.30am - Future Year 2033 - Cooper Road / Moriarty Way

																					-				
Vehicle Mov Mov Turn ID	ehicle Movement Performance lov Turn DEMAND ARRIV TOWS FLOW	rformance ARRIVAL FLOMS	25 S		Aver Level of Delay Service	₩ ₩	AGE BACK FOURJE	£ 8	58	ahver No. A Oydes Spa	Aner	Vehici Mov_	e Mover Turn	cle Movement Performance Turn DEMAND ARRIV FLOWS FLOW	rforman ARB FE	ARRIVAL FLOWS	g je	Aver. L Delay	Level of Service	AVERAGE BACK OF QUEUE	EBACK	Page B	Effective Ave	Aver. No. Aver.	M M
	Fotal How	V] [Total HV]	¥ 5-		, i	<u>P</u>	E Det		Rate		funh		_ *	いまれ	-0.0	E SE	¥	×		<u> </u>	E E		Rage Ballet		ŧ
South: Cooper Road	yr Road											South	South: Cooper Road	Poad											
2 T1		33					2.5	0.23			39.5						0.170	52	LOSA	0.4	2.7	0.24	0.52	0.24	39.5
3 82	40 0	0.0 40 0.0	0 0.161	1 8.1	1 LOSA	A 0.4	2.5	0.23	0.52	0.23 3	38.8	3	8		0.0 40		0.170	9.1	LOSA	0.4	2.7	0.24	0.52	0.24	38.8
Approach	193 0.	0.0 193 0.0	0 0.161	1 5.8	8 LOSA	A 0.4	25	023	0.52	0.23	39.3	Approach		203 0	0.0 203	3 0.0	0.170	5.8	LOSA	0.4	27	0.24	0.52	0.24	39.3
East Moniarty Way	/ Way											East A	East Morianty Way	(a)											
4 12	38 0.	0.0 38 0.0	0 0.100	0 5.5	5 LOSA			0.32	0.61	0.32	38.5	4	7	38 0	0.0 38	00	0.104	55	LOSA	0.2	1.4	0.33	0.62	0.33	38.5
6 R2		000 29 000	0 0 100	0 8.3	3 LOSA	A 0.2	1.4	0.32	190	0.32	26.8	9			69 00	0.0	0.104	8.4	LOSA	0.2	1.4	0.33	0.62	0.33	26.7
Approach	105 0.	0.0 105 0.0	0 0.100	0 7.3	3 LOSA			0.32		0.32 3	33.0	Approach		107 0.	0.0 107	0.0 7	0.104	7.4	LOSA	0.2	4	0.33	0.62	0.33	32.8
North: Cooper Road	r Road											North:	North: Cooper Road	bead											
7 12	44 0	0.0 44 0.0	0 0.146	5.1	1 LOSA	A 0.3		0.17	0.49	0.17 4	46.3	7	7		0.0 45	0.0	0.154	5.1	LOSA	0.3	2.3	0.17	0.49	0.17	46.3
8 T1	141 0.	0.0 141 0.0	0 0.146	9 9	0 LOSA	A 0.3	22	0.17	0.49	0.17 5	53.1	8		152 0	0.0 152	0.0 2	0.154	9.0	LOSA	0.3	23	0.17	0.49	0.17	53.1
Approach	186 0.	0.0 186 0.0	0 0.145	9 20	O LOSA	A 0.3	22	0.17	0.49	0.17 5	51.5	Approach		0 261	761 0.0	0.0 7	0.154	5.0	LOSA	0.3	23	0.17	0.49	0.17	515
All Vehicles	484 0.	0.0 484 0.0	0 0.161	1 5.8	8 LOSA	A 0.4	2.5	023	0.63	023 4	46.5	All Vehicles		909	905 000	8 0.0	0.170	5.8	LOSA	0.4	2.7	0.23	0.53	0.23	46.5

MOVEMENT SUMMARY - 2.45pm - 3.45pm - Future Year 2033 - Bagdad Street / Auburn Road

			_	Pre-	Dev	Pre-Developm	ment	¥									Post	-De	velo	Post-Development	Ħ				
e Mov	Vehicle Movement Performance	Perform	ance										Vehicle N	ehicle Movement Performance	t Perfor	mance									
Ę.	DEMAND FLOWS (Total HV)		ARRIVAL FLOWS [Total HV]	Sath y	Aver. Level of Delay Service sec	evel of Service	AVERAGE OF QUE Veh.	RAGE BACK of QUEUE sh. Dist] h m	Prop. Oue. Eff	Effective Aver. No. Stop Cycles Rate	S	wer. seed	Mov Tum ID		WS WS	ARRIVAL FLOWS [Total HV]	Satur Satur	Aver. L Delay 3	Level of Service	AVERAGE BACK OF QUEUE [Veh. Dist] veh. m	BACK Bet]	Prop. Oue. Ell	EffectiveAver. No. Stop Cycles : Rate	00	Awer.
South: Aubum Road	Road												South: Au	South: Auburn Road											
= :	393			0.203		LOSA	0.0	000	000			6.69			0.0		0.203		LOSA	0.0	0.0	00.00	000		6.69
82	33	00	32 0.0	0.039	7.0	LOSA	0.1	0.4	0.58	0.73	0.58	20.4	3	7 40	00	40 0.0	0.050	7.1	LOSA	0.1	9.0	0.59	0.75	0.59	20.3
Approach	424	0.0	424 0.0	0.203	0.5	¥	0.1	0.4	500	900	0.04 5	27.7	Approach	433	0.0	433 0.0	0.203	0.7	N	0.1	9.0	900	0.07	90.09	57.1
East: Bagdad Street	Street												East Bagdad Street	dad Street											
7	8	00	28 0.0	0.678	16.2	LOSB	1.5	10.2	0.85	121	1.62	38.5	4 12	30	0.0	30 0.0	0.716	17.3	10SB	1.6	11.4	98.0	125	177	37.7
82	217	000	217 0.0	829.0	24.8	E SOI	1.5	102	987	121	1.62 4	40.8	6 R2	226	0.0	226 0.0	0.716	26.2	108B	1.6	11.4	98.0	125	1.77	40.1
Approach	245	000	245 0.0	0.678	23.8	108B	1.5	102	987	121	1.62 4	40.6	Approach	256	0.0	256 0.0	0.716	25.1	108B	1.6	11.4	98'0	125	1.77	39.8
North: Aubum Road	Road												North: Aubum Road	burn Road											
2	75	0.0	254 0.0	0.367	9.6	LOSA	0.0	0.0	00.00	021	0.00	512	7 12	556	0.0	256 0.0	0.368	9.6	LOSA	0.0	0.0	000	0.22	000	51.1
Ξ	448	0.0	448 0.0	0.367		LOSA	0.0	0.0	000	021	0.00	52.7	8 T	448	0.0	448 0.0	0.368	0.0	LOSA	0.0	0.0	000	0.22	000	52.7
Approach	700		702 0.0	0.367	2.0	¥	0.0	0.0	0000	021 (0.00 5	522	Approach	705	0.0	705 0.0	0.368	2.1	N	0:0	0.0	000	0.22	00:00	52.2
All Vehicles	1372	0.0	1372 0.0	8290	5.5	N	1.5	102	0.16	0.34	0.30 4	47.2	All Vehicles	es 1395	0.0	1395 0.0	0.716	5.9	¥	1.6	11.4	0.18	0.36	0.34	46.5

MOVEMENT SUMMARY - 2.45pm - 3.45pm - Future Year 2033 - Cooper Road / Moriarty Way

	kmħ	km/h 0.04 41.5			_	_						
Zelle	ı	0.53	0.53	0.53 0.53 0.53	0.53	0.53	0.53	0.53 0.53 0.55 0.56 0.56	0.53 0.53 0.55 0.55 0.56	0.53 0.53 0.55 0.56 0.56	0.53 0.53 0.55 0.56 0.56 0.50 0.50	0.53 0.53 0.55 0.56 0.56 0.50
	ı	0.04	0.04	20.00 20.00 20.00	90.0 90.0 90.0	0.04	0.04 0.04 0.28 0.28	0.04 0.04 0.04 0.28 0.28	0.04 0.04 0.09 0.28 0.28	0.04 0.04 0.28 0.28 0.28	0.04 0.04 0.28 0.28 0.11 0.11	0.04 0.04 0.28 0.28 0.11 0.11
weh. Ust		0.2 1.3	02 13 02 13	02 13 02 13 02 13	02 13 02 13 02 13	02 13 02 13 02 13 01 04	02 1.3 02 1.3 02 1.3 0.1 0.4 0.1 0.4	02 13 02 13 02 13 02 04 01 04	02 13 02 13 02 13 01 04 01 04	02 13 02 13 02 13 01 04 01 04 01 04	02 13 02 13 02 13 02 13 01 04 01 04 01 04	02 13 02 13 02 13 01 04 01 04 01 04 01 04 01 04
		LOSA	LOSA	LOSA LOSA LOSA	LOSA LOSA LOSA	LOSA LOSA LOSA LOSA	LOSA LOSA LOSA LOSA	LOSA LOSA LOSA LOSA LOSA	LOSA LOSA LOSA LOSA LOSA	LOSA LOSA LOSA LOSA LOSA LOSA	LOSA LOSA LOSA LOSA LOSA LOSA LOSA LOSA	LOSA LOSA LOSA LOSA LOSA LOSA LOSA LOSA
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							112 24 136 136 5	112 24 136 136 33	112 24 24 24 33 33	112 24 24 24 28 28 5 5 6 6 6 6 6	112 136 136 137 137 138 139 139 139 139 139 139 139 139 139 139	112 24 136 138 33 131 191 191
	Road	Road 112 0.0			2	78	20	28	_	_	_	_
_, >	uft: Cooper	Souffi: Cooper Road 2 T1 112	ufti: Cooper I T1 R2	South: Cooper 2 2 T1 3 R2 Approach	Souffi: Cooper Ros 2 T1 112 3 R2 24 Approach 136 East Moriarty Way	Afri. Cooper! T1 R2 R2 roach tt Morianty V	Iffi: Cooper! T1 R2 roach tt Morianty V R2 R2	South: Cooper I 2 T1 3 R2 Approach Approach 4 L2 6 R2 Approach	Souffi: Cooper Road 2	Mr. Cooper Int. Co	Afr. Cooper Int. C	Souft: Cooper I 3 R2 3 R2 Approach L2 6 R2 Approach North: Cooper F 7 L2 7 L2 Approach Approach
	S	S _o	3 2 S	3 2 g	Ba Ap	8 2 S 8	8 2 E 4 B 4 9	8 2 E B 4 9 4	8 4 8 9 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8	8	8 4 P P P P P P P P P P P P P P P P P P	8 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4
	kmy	km/h 41.5	41.5 40.1	41.5 40.1 41.1	41.5 40.1 41.1	41.5 40.1 41.1 40.6	41.5 40.1 41.1 40.6 29.2	41.5 40.1 41.1 40.6 40.0	41.5 40.1 40.6 40.6	41.5 40.1 41.1 41.1 40.0 40.0	41.5 40.1 40.1 40.0 40.0 40.0 53.3	41.5 40.1 41.1 40.6 29.2 40.0 40.0 53.3 50.9
		200										
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E 6		0.2 1.	02 1	0.2 1.0	0.2 1.	02 1. 0. 0.1	02 1. 02 1. 02 1. 01 0	002 11 00 01 0	0.2 1. 0.0 0.1 0.0 0.0	00 1.0 0.0 0.1	002 1.1 002 1.1 001 0.0 001 0.0 003 1.1 003 1.1	002 1-1 002 1-1 001 00 01-1 003 1-1 003 1-1 003 1-1
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Till fan		OSA	LOSA	LOS A LOS A LOS A	OS A LOS A LOS A	0S A 10S A 10S A	OSA OSA OSA OSA	00 A 00 A 00 A 00 A 00 A 00 A	8 8 8 8 8			
		4.8 LOSA										4.8 7.7 5.4 5.3 8.1 5.0 5.0 6.0 6.0
ve sec vet		0.084 4.8	0.084 4.8	0.084 4.8 0.084 7.7 0.084 5.4	0.084 4.8 0.084 7.7 0.084 5.4	0.084 4.8 0.084 7.7 0.084 5.4 0.029 5.3	0.084 4.8 0.084 7.7 0.084 5.4 0.029 5.3 0.029 8.1	0.084 48 0.084 77 0.084 5.4 0.029 5.3 0.029 8.1	0.084 4.8 0.084 7.7 0.084 5.4 0.029 5.3 0.029 8.1	0.084 4.8 0.084 7.7 0.084 5.4 0.029 5.3 0.029 8.1 0.029 5.6	0.084 48 0.084 77 0.084 5.4 0.029 5.3 0.029 8.1 0.029 5.6 0.134 5.0	0.084 0.084 0.084 0.029 0.029 0.029 0.034 0.134
<u> </u>		104 0.0 0.084 4.8	104 0.0 0.084 4.8 24 0.0 0.084 7.7	104 0.0 0.084 4.8 24 0.0 0.084 7.7 128 0.0 0.084 5.4	104 0.0 0.064 4.8 24 0.0 0.064 7.7 128 0.0 0.064 5.4	104 0.0 0.084 4.8 24 0.0 0.084 7.7 128 0.0 0.084 5.4 28 0.0 0.029 5.3	104 0.0 0.084 4.8 24 0.0 0.084 7.7 128 0.0 0.084 5.4 28 0.0 0.029 5.3 4 0.0 0.029 8.1	104 00 0084 48 24 00 0084 77 128 00 0084 54 28 00 0029 53 4 00 0029 81 32 00 0029 56	104 00 0064 4.8 24 00 0064 7.7 128 0.0 0084 5.4 28 0.0 0029 5.3 4 0.0 0029 8.1 32 0.0 0029 5.6	104 00 0064 4.8 24 00 0064 7.7 128 00 0084 5.4 28 00 0029 5.3 4 00 0029 8.1 32 00 0029 5.6 64 00 0.134 5.0	104 00 0064 4.8 24 00 0064 77 128 00 0084 5.4 28 00 0029 5.3 4 00 0029 8.1 32 00 0029 5.6 64 00 0.134 5.0 116 00 0.134 4.9	104 0.0 0.084 24 0.0 0.084 128 0.0 0.084 4 0.0 0.029 32 0.0 0.029 32 0.0 0.029 64 0.0 0.134 116 0.0 0.134
avi seconi % vehil % vic sec		0.0 104 0.0 0.084 4.8	0.0 104 0.0 0.084 4.8 0.0 24 0.0 0.084 7.7	0.0 104 0.0 0.084 4.8 0.0 24 0.0 0.084 7.7 0.0 128 0.0 0.084 5.4	0.0 104 0.0 0.084 4.8 0.0 24 0.0 0.084 7.7 0.0 128 0.0 0.084 5.4	00 104 00 0.084 4.8 0.0 24 0.0 0.084 7.7 0.0 128 0.0 0.084 5.4 0.0 28 0.0 0.029 5.3	00 104 00 0.084 4.8 0.0 24 0.0 0.084 7.7 0.0 128 0.0 0.084 5.4 0.0 28 0.0 0.029 5.3 0.0 4 0.0 0.029 8.1	00 104 0.0 0.084 4.8 0.0 24 0.0 0.084 77 0.0 128 0.0 0.084 5.4 0.0 28 0.0 0.029 5.3 0.0 4 0.0 0.029 8.1 0.0 32 0.0 0.029 5.6	00 104 00 0084 4.8 00 24 00 0084 7.7 00 128 00 0.084 7.7 00 28 00 0.029 5.3 00 4 00 0.029 8.1 00 32 00 0.029 5.6	00 104 00 0084 4.8 00 24 00 0084 7.7 00 128 00 0084 7.7 00 28 00 0029 5.3 00 4 00 0029 5.3 00 32 00 0029 5.6	00 104 00 0.084 4.8 0.0 24 0.0 0.084 7.7 0.0 128 0.0 0.084 7.7 0.0 28 0.0 0.029 5.3 0.0 4 0.0 0.029 8.1 0.0 32 0.0 0.029 5.6 0.0 64 0.0 0.134 5.0 0.0 116 0.0 0.134 4.9	00 104 00 0084 00 24 00 0084 00 128 00 0089 00 28 00 0029 00 32 00 0029 00 32 00 0029 10 64 00 0.134 00 116 00 0.134
3	130 TA R 11854 R	per Road 104 0.0 104 0.0 0.084 4.8	0.0 104 0.0 0.084 4.8 0.0 24 0.0 0.084 7.7	00 104 00 0.084 4.8 00 24 00 0.084 7.7 00 128 00 0.084 5.4	Per Road 104 0.0 104 0.0 0.084 4.8 24 0.0 24 0.0 0.084 7.7 128 0.0 128 0.0 0.084 5.4	Per Road 104 0.0 104 0.0 0.084 4.8 24 0.0 24 0.0 0.084 7.7 128 0.0 128 0.0 0.084 5.4 rty Way 28 0.0 28 0.0 0.029 5.3	Per Road 104 0.0 104 0.0 0.084 4.8 24 0.0 24 0.0 0.084 7.7 128 0.0 128 0.0 0.084 5.4 rty Way 28 0.0 28 0.0 0.029 5.3 4 0.0 4 0.0 0.029 8.1	Per Road 144 0.0 104 0.0 0.064 4.8 24 0.0 24 0.0 0.064 7.7 128 0.0 128 0.0 0.064 5.4 rry Way 4 0.0 28 0.0 0.029 8.1 32 0.0 32 0.0 0.029 8.1	per Road 104 00 104 00 0.084 4.8 124 0.0 24 0.0 0.084 7.7 128 0.0 128 0.0 0.084 5.4 14 0.0 28 0.0 0.029 5.3 28 0.0 32 0.0 0.029 5.6 per Road	per Road 104 00 104 00 0.084 4.8 124 0.0 24 00 0.084 7.7 128 0.0 128 0.0 0.084 5.4 14 0.0 28 0.0 0.029 5.3 28 0.0 32 0.0 0.029 5.6 per Road 64 0.0 64 0.0 0.134 5.0	Per Road 104 00 104 00 0.084 4.8 24 0.0 24 0.0 0.084 7.7 128 0.0 128 0.0 0.084 5.4 rry Way 4 0.0 28 0.0 0.029 8.1 32 0.0 32 0.0 0.029 5.5 Per Road 64 0.0 64 0.0 0.134 5.0 116 0.0 116 0.0 0.134 4.9	per Road 104 00 104 00 0.084 124 0.0 24 0.0 0.084 128 0.0 128 0.0 0.084 29 0.0 28 0.0 0.029 4 0.0 4 0.0 0.029 32 0.0 32 0.0 0.029 per Road 64 0.0 64 0.0 0.134 116 0.0 116 0.0 0.134